ROOF/COLUMN LAYOUT

SINGLE SPAN ONLY

ROOF OVER OPEN STRUCTURE

NO WALL OR SCREEN COMPONENTS

(OPTIONAL **CONNECTION TO** WALL, SEE DETAIL)

COLUMN PER TABLE OR SITE-SPECIFIC **ENGINEERING**

EVE CARRY BEAM PER TABLES

OR SITE-SPECIFIC ENGINEERING



POSITIVE AND NEGATIVE DESIGN PRESSURES CALCULATED FOR USE WITH THIS SYSTEM SHALL BE DETERMINED BY OTHERS ON A JOB-SPECIFIC BASIS IN ACCORDANCEWITH ASCE 7-16 AND CHAPTER 16 OF THE 2017 FLORIDA BUILDING ACCORDANCEWITH ASCE 7-16 AND CHAPTER 16 OF THE 2017 FLORIDA BOLLDING CODE (6TH EDITION), THE 2020 FLORIDA BUILDING CODE (7TH EDITION) AND THE 2012/2015/2018 IBC/IRC, AS WELL AS CURRENT VERSIONS OF THE MN, NC, NJ, NY, OH, SC, & VA BUILDING CODES AS APPLICABLE. CODE ENFORCED COMPLIES WITH STATE OF SEAL AND IF MULTIPLE VERSIONS LISTED THEN MOST STRINGENT

DESIGN SHALL UTILIZE ASD DESIGN METHOD USING ASCE 7-16 OR ASCE 7-10 BASED ON THE APPLICABLE CODE USED.

*THIS DOCUMENT DOES NOT CERTIFY PRODUCT FOR USE AS A HABITABLE STRUCTURE. AAMA 2100 SUNROOM CLASSIFICATION II, III, OR IV ONLY.

NO 33-1/3% INCREASE IN ALLOWABLE STRESS HAS BEEN USED IN THE DESIGN OF THIS SYSTEM. WIND LOAD DURATION FACTOR Cd=1.6 HAS BEEN USED FOR WOOD

ALL FASTENERS TO BE #8 OR GREATER SAE GRADE 5 UNLESS NOTED OTHERWISE. FASTENERS SHALL BE CADMIUM-PLATED OR OTHERWISE CORROSION-RESISTANT MATERIAL AND SHALL COMPLY WITH "SPECIFICATIONS FOR ALUMINUM STRUCTURES" SECTION 5.2.1 BY THE ALUMINUM ASSOCIATION, INC., & ANY APPLICABLE FEDERAL, STATE, AND/OR LOCAL CODES.

ALL TAPCONS MUST BE ITW /DEWALT CARBON STEEL TAPCONS OR EQUIVALENT W/ 1-3/4" EMBED, 3" MIN. EDGE DISTANCE, FASTENED TO MINIMUM 3KSI MIN.,

10. THE CONTRACTOR IS RESPONSIBLE TO INSULATE ALL MEMBERS FROM DISSIMILAR

MATERIALS TO PREVENT ELECTROLYSIS.

11. ALL ALUMINUM SHALL BE 6063-T6 ALLOY AND TEMPER UNLESS NOTED OTHERWISE.

12. SITE SPECIFIC ENGINEERING REQUIRED FOR ANY DETAIL WHICH DEVIATES FROM THIS PLAN OR BEYOND THESE LIMITATIONS.

13. ENGINEER SEAL AFFIXED HERETO VALIDATES STRUCTURAL DESIGN AS SHOWN ONLY. USE OF THIS SPECIFICATION BY CONTRACTOR, et. al. INDEMNIFIES & SAVES HARMLESS THIS ENGINEER FOR ALL COST & DAMAGES INCLUDING LEGAL FEES & APPELLATE FEES RESULTING FROM MATERIAL FABRICATION, SYSTEM ERECTION, & CONSTRUCTION PRACTICES BEYOND THAT WHICH IS CALLED FOR BY LOCAL, STATE

CONSTRUCTION PRACTICES BEYOND THAT WHICH IS CALLED FOR BY LOCAL, STATE, & FEDERAL CODES & FROM DEVIATIONS OF THIS PLAN.

14. ELECTRICAL GROUND AND ALL RELATED WIRING AND CONSIDERATIONS TO BE DESIGNED BY OTHERS AS REQUIRED.

15. THE SYSTEM DETAILED HEREIN IS GENERIC AND DOES NOT PROVIDE INFORMATION FOR A SPECIFIC SITE. FOR SITE CONDITIONS DIFFERENT FROM THE CONDITIONS DETAILED HEREIN, A LICENSED PROFESSIONAL SHALL PREPARE SITE SPECIFIC DOCUMENTS FOR LISE IN CONJUNCTION WITH THIS DOCUMENT.

AFFIRMATIONS ARE INTENDED.

PERMITTED AND INVALIDATE THIS CERTIFICATION.

FOR ENGINEER CERTIFIED ORIGINALS & MORE INFORMATION ABOUT THIS DOCUMENT OR SCAN THE QR CODE TO THE RIGHT >

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GENERAL NOTES:

THE DESIGN. THE PROJECT SUPERSTRUCTURE WITH WHICH THIS DESIGN IS USED SHALL BE RESPONSIBLE FOR THE INTEGRITY OF ALL SUPPORTING SURFACES TO THIS DESIGN WHICH SHALL BE COORDINATED BY THE PERMITTING CONTRACTOR.

ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURERS' RECOMMENDATIONS. MINIMUM EMBEDMENT SHALL BE AS NOTED HEREIN. MINIMUM EMBEDMENT AND EDGE DISTANCE EXCLUDES STUCCO, FOAM, BRICK,

ALL CONCRETE ANCHORS SHALL BE INSTALLED TO NON-CRACKED CONCRETE ONLY.

ALL CONNECTING WOOD MEMBERS SHALL BE G=0.55 MIN PRESSURE TREATED

MATERIALS TO PREVENT ELECTROLYSIS.

DOCUMENTS FOR USE IN CONJUNCTION WITH THIS DOCUMENT.

16. EXCEPT AS EXPRESSLY PROVIDED HEREIN, NO ADDITIONAL CERTIFICATIONS OR

17. ALTERATIONS, ADDITIONS OR OTHER MARKINGS TO THIS DOCUMENT ARE NOT

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EXISTING

STRUCTURE

(BY OTHERS)

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BAL @ 14" MAX. O.C. STAGGERED* RECEIVING *NOTE: USE (2) ROWS ¼"x1¾" EMBED TAPCONS, 2½" & 4½" FROM ENDS, 12" MAX O.C. STAGGERED

SEAL JOINT CONTINUOUS

W/ CAULKING OR

SEALING TAPE

TO HOLLOW BLOCK

FASTEN RECEIVING CHANNEL TO 2" SOUND FASCIA WITH (2) ROWS X"x2" LAG BOLTS 2" FROM ENDS & SPACED AS FOLLOWS (STAGGERED): FOR TOTAL LOADS ≤ 30 PSF; 5" O.C FOR TOTAL LOADS < 40 PSF: 3" O.C. RECEIVING CHANNEL

(%" MIN THICKNESS)

#14x¾" SMS AS FOLLOWS PER 4' PANEL TOP &

FACIA MINIMUN

ROOF SPAN FROM TABLE MIN ROOF PITCH IS ¾" PER

12", MAX IS 2" IN 12'

ADD (2) SMS 2" & 4" FROM EACH ENCLOSURE END TOP & BOTTOM. HEAVY ADHESIVE CAULK OR SEALING TAPE TOP & BOTTOM CONTINUOUS

FOR TOTAL LOADS \leq 30: (8) AT 7" O.C. FOR TOTAL LOADS \leq 40: (16) AT 3.5" O.C.

CONNECTING BEAM TO WAI ROOF CONNECTION TO CONNECTING BEAM TO WALL) CARRY BEAM PER DETAIL 6/2 CARRY BEAM PER TABLES OR SEALED SITE (2) SIDE ANGLES TO CARRY BEAM W/ (2) ¾6" THRUBOLTS SPECIFIC DRAWINGS/CALCULATIONS ANGLE DIMENSIONS AS REQUIRED (MIN. 1/8" THICK) SIDE ANGLES TO HOST STRUCTURE AS FOLLOWS. (1) BOTTOM ANGLE TO CARRY BEAM W/ (5) #14 SMS. TO CONCRETE/BLOCK:

BOTTOM ANGLE TO HOST STRUCTURE AS FOLLOWS TO CONCRETE/BLOCK:

(2) $\%_6$ "x1%" EMBED ITW (XL) TAPCONS, WITH 2%" MIN SPACING AND 4" MIN EDGE DIST

(3) #14x2" EMBED WOOD SCREWS WITH 1" MIN SPACING, 0.75" MIN. EDGE DISTANCE (2) ¾s"x1¾" EMBED ITW (XL) TAPCONS PER ANGLE, WITH 2¾" MIN SPACING AND 4" MIN EDGE DIST.

#14x2" EMBED WOOD SCREWS WITH 1" MIN SPACING, 0.75" MIN. EDGE DISTANCE:

FOR TOTAL LOADS ≤30 PSF: (4) PER ANGLE

• FOR TOTAL LOADS ≤40 PSF: (6) PER ANGLE

OPTIONAL EAVE BEAM CONNECTION AT WALL

 ¼" LAGS (REQUIRED ONLY IF OPTIONAL SIDE BEAM USED), 4" FROM ENDS & 24" MAX. O.C.,
 1" MIN LONGER THAN PANEL THICKNESS TO TYP. TOP & SECURE ROOF TO NON-PRIMARY WALLS ONLY CAULK ALL SCREW HEADS воттом COMPOSITE ROOF PANEL END CAP SIDE BEAM

ANGLE (REQUIRED ONLY IF OPTIONAL BEAM USED), WITH (7) #14 SMS PER LEG, PER SIDE OF

FOR SNOW LOADS <30 PSF: (5) #14 SMS FOR SNOW LOADS <60 PSF: (10) #14 SMS PER BEAM, PER PLATE. **EVE CARRY BEAM** 6" WIDE X 1/8" THICK-ALUM PLATE ON EACH SIDE OF CARRY BEAM COLUMN PER TABLES OR SEALED SITE -

(1) 1/4" LAG BOLT INTO EACH CARRY BEAM END, 1" MIN LONGER THAN

SPECIFIC DRAWINGS/CALCULATIONS

SECTION AT SIDE OF ROOF PANEL (OPTIONAL)

-COLUMN PER TABLES OR SEALED SITE SPECIFIC DRAWINGS/CALCULATIONS

#12x¾" SMS @ 16" TYP. TOP & BOTTOM **GUTTER**₁ (OPT.) EVE CARRY BEAM PER TABLES OR SEALED SITE SPECIFIC OPTION 1: ¼" LAG BOLTS AT 11"
O.C., MIN. (4)PER 48" WIDE-PANEL, W/ 1" MIN. WASHER DRAWINGS/CALCULATIONS BOTH OPTIONS: (2) %" THRUBOLTS, CARRY BEAM TO COLUMN DIAMETER, 1" MIN. LONGER THAN PANEL THICKNESS NOTCH COLUMN IN CENTER TO FIT CARRY BEAM SNUG OPTION 2: 1/4" THRUBOLTS-THROUGH ROOF, 4" FROM ENDS BOTH OPTIONS: 3"x3"x╬" ANGLE, (6) #14 SMS EA LEG, EA SIDE OF COLUMN & 17" O.C., W/ 1" MIN WASHER DIAMETER, TO 2"x2"x\%" ANGLES (EITHER SIDE IF COLUMN PER TABLES OR SEALED SITE REQUIRED BY BUILDING DEPT). SPECIFIC DRAWING/CALCULATIONS

SECTION AT BEARING COLUMN

SEALED SITE SPECIFIC DRAWINGS/CALCULATIONS

COLUMN PER TABLES OR-

INTERNAL BASE SHOE W/ 2½" FLANGES TO FIT WITHIN COLUMN. ANCHOR TO SLAB W/ (4) ½"x1¾" EMBED. TAPCONS W/ ½" MIN. WASHER DIAMETER , 21/2" APART & 3" FROM SLAB EDGE INSIDE COLUMN. ANCHOR COLUMN TO INTERNAL FLANGES W/ (6) #14 SMS, ½" FROM

BASE EDGES, ON EACH SIDE OF COLUMN

-3"x3"x3"x⅓" ANGLE WITH (2) ¼"x1¾" EMBED.

TAPCONS W/ 5/8" MIN. WASHER DIAMETER TO

SLAB, 21/8" APART & 3" FROM SLAB EDGE & (6)

#14 SMS TO COLUMN, 1/2" FROM ANGLE EDGES,

DRAWINGS/CALCULATIONS INTERNAL BASE SHOE W/ 21/8" FLANGES TO FIT WITHIN COLUMN. ANCHOR TO SLAB ON BOTH SIDES W/ (2) ¼"x1¾" EMBED. TAPCONS W/ 3" MIN. WASHER DIAMETER 2½" APART & 3" FROM SLAB EDGE, OUTSIDE OF COLUMN. ANCHOR COLUMN TO INTERNAL

COLUMN PER TABLES OR ·

SEALED SITE SPECIFIC

FLANGES W/ (6) #14 SMS, ½" FROM BASE EDGES, ON EACH SIDE OF COLUMN

ON EACH SIDE OF COLUMN

COLUMN CONNECTION TO DECK OPTIONS

EMAIN IN DIGITAL FORMAT, SHAL VERIFIED BY ELECTRONIC MEANS

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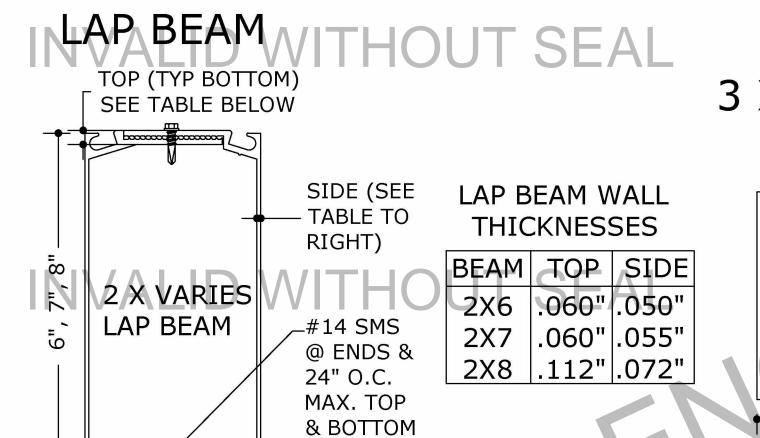
SEASONS / METALS OVER OPEN STRUCT

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EXTRUSIONS:



3 X 3 SQUARE **TUBE**

.090" OR .125" 3 X 3 SQUARE TUBE 3" SQUARE

ANGLES

DEPTH AS REQUIRED (2" MIN.)

2" OR 3"

NOTE: VERIFY FIELD EXTRUSION DIMENSIONS WITH DESIGN DIMENSIONS PRIOR TO USE. ALTERNATE ENGINEERING WILL BE REQUIRED FOR DIMENSIONS LESS THAN THOSE AS SPECIFIED HEREIN. ALL EXTRUSIONS TO BE 6063-T6 ALUMINUM.

INVALID WITHOUT SEAL INVALID WITHOUT SEA

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COLUMN ALLOWABLE HEIGHT TABLES:

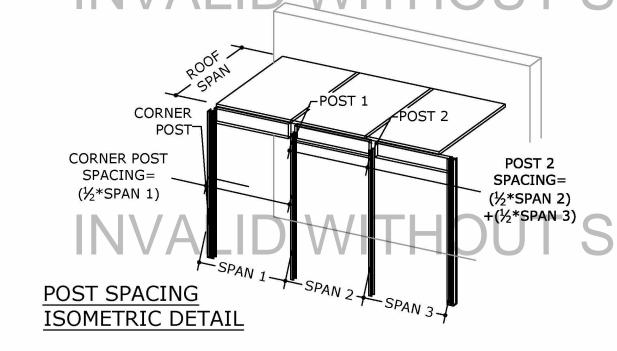
COLUMN	IMN MAX ROOF LIVE LOAD AVERAGE COLUMN SPACING W(FT)							
TYPE	SPAN S (FT)	GRAVITY (PSF)	6	8	10	12	14	15
			10'-0''	10'-0"	10'-0''	10'-0''	10'-0"	10'-0''
			10'-0"	10'-0"	10'-0''	10'-0"	10'-0"	10'-0''
	/ I /	10	10'-0"	10'-0"	10'-0"	10'-0"	10'-0"	10'-0''
	$\mathbb{N} \mathbb{N}$	\prime \triangle \mid	10'-0"	10'-0"	10'-0''	10'-0''	10'-0''	10'-0''
	1 4 V	/	10'-0"	10'-0"	10'-0''	10'-0''	10'-0"	10'-0"
			10'-0"	10'-0''	10'-0''	10'-0''	10'-0"	10'-0''
		20	10'-0"	10'-0''	10'-0''	10'-0''	10'-0"	10'-0''
	10		10'-0"	10'-0''	10'-0''	10'-0''	9'-8"	9'-3''
	10		10'-0"	10'-0''	10'-0''	10'-0''	10'-0"	10'-0''
		20	10'-0"	10'-0"	10'-0''	10'-0"	10'-0"	10'-0''
		30	10'-0"	10'-0''	10'-0''	10'-0''	9'-6''	8'-11''
			10'-0"	10'-0"	10'-0''	9'-9''	8'-6"	8'-0''
			10'-0"	10'-0''	10'-0''	10'-0''	10'-0"	10'-0''
×		40	10'-0"	10'-0''	10'-0''	10'-0''	9'-5"	8'-8''
G 3x3x.093 BOX		40	10'-0"	10'-0"	10'-0''	9'-8''	8'-2"	7'-6''
93			10'-0"	10'-0''	10'-0''	8'-8''	7'-4''	6'-9''
×.	N I N	10	10'-0''	10'-0"	10'-0''	10'-0''	10'-0"	10'-0''
æ			10'-0"	10'-0"	10'-0''	10'-0''	10'-0"	10'-0''
ט	$\cup V$		10'-0"	10'-0''	10'-0''	10'-0''	10'-0"	10'-0''
			10'-0"	10'-0''	10'-0''	10'-0''	9'-11"	9'-5''
			10'-0"	10'-0''	10'-0''	10'-0''	10'-0"	10'-0''
		20	10'-0"	10'-0''	10'-0''	10'-0''	10'-0''	9'-3''
			10'-0"	10'-0''	10'-0''	10'-0''	8'-7"	8'-0''
	20		10'-0"	10'-0''	10'-0''	9'-0''	7'-8''	7'-2''
	20		10'-0''	10'-0''	10'-0''	10'-0''	8'-8''	7'-8''
		30	10'-0"	10'-0''	10'-0''	9'-0''	7'-1''	6'-3''
			10'-0''	10'-0''	9'-9''	7'-9''	6'-2''	5'-5''
	_		10'-0"	10'-0''	8'-9''	7'-0''	5'-6''	4'-10''
		40	10'-0"	10'-0''	10'-0''	7'-9''	5'-2''	4'-1''
			10'-0"	10'-0''	8'-10''	6'-4''	4'-3''	₽
		70	10'-0"	10'-0''	7'-8''	5'-6''	-	-
		/ A	10'-0"	9'-4"	6'-10"	5'-0''	1	-

	COLUMN	MAX ROOF	LIVE LOAD	AVERAGE COLUMN SPACING W(FT)						
	TYPE	SPAN S (FT)	GRAVITY (PSF)	6	8	10	12	14	15	
			10	10'-0"	10'-0''	10'-0''	10'-0''	10'-0''	10'-0''	
				10'-0"	10'-0''	10'-0''	10'-0''	10'-0''	_ 10'-0''	
				10'-0"	10'-0"	10'-0''	10'-0''	10'-0"	10'-0''	
SEA				10'-0"	10'-0"	10'-0''	10'-0''	10'-0''	10'-0''	
	\ <u> </u>			10'-0''	10'-0''	10'-0''	10'-0''	10'-0''	10'-0''	
				10'-0"	10'-0''	10'-0''	10'-0''	10'-0''	10'-0''	
			20	10'-0"	10'-0"	10'-0''	10'-0''	10'-0''	10'-0''	
		10		10'-0"	10'-0''	10'-0''	10'-0''	10'-0''	10'-0''	
				10'-0"	10'-0"	10'-0''	10'-0''	10'-0''	10'-0''	
			30	10'-0"	10'-0"	10'-0''	10'-0''	10'-0''	10'-0'	
				10'-0"	10'-0"	10'-0''	10'-0''	10'-0''	10'-0''	
				10'-0"	10'-0"	10'-0''	10'-0''	10'-0''	10'-0''	
	вох		40	10'-0"	10'-0''	10'-0''	10'-0''	10'-0"	10'-0'	
				10'-0"	10'-0"	10'-0''	10'-0''	10'-0"	10'-0'	
				10'-0"	10'-0"	10'-0''	10'-0''	10'-0"	10'-0''	
	25			10'-0"	10'-0"	10'-0''	10'-0''	9'-7"	9'-0''	
	G 3x3x.125 BOX		10	10'-0"	10'-0''	10'-0''	10'-0''	10'-0"	_ 10'-0'	
SEA				10'-0''	10'-0"	10'-0''	10'-0''	10'-0"	10'-0"	
$\mathcal{S} \vdash \mathcal{A}$				10'-0"	10'-0"	10'-0''	10'-0"	10'-0''	10'-0'	
				10'-0"	10'-0"	10'-0''	10'-0''	10'-0"	10'-0''	
		1	20	10'-0"	10'-0"	10'-0''	10'-0"	10'-0''	10'-0'	
				10'-0"	10'-0"	10'-0"	10'-0''	10'-0''	10'-0''	
				10'-0"	10'-0"	10'-0''	10'-0''	10'-0''	10'-0''	
			1	10'-0"	10'-0"	10'-0''	10'-0''	10'-0''	9'-4"	
		20		10'-0"	10'-0"	10'-0''	10'-0''	10'-0''	10'-0''	
				10'-0"	10'-0''	10'-0''	10'-0''	10'-0''	9'-6''	
			30	10'-0"	10'-0"	10'-0''	10'-0''	9'-0"	8'-3"	
				10'-0"	10'-0"	10'-0''	9'-7''	8'-0"	7'-4''	
			1.0	10'-0"	10'-0"	10'-0''	10'-0''	9'-7"	8'-5"	
				10'-0"	10'-0"	10'-0''	10'-0''	7'-10''	6'-11''	
			40	10'-0"	10'-0''	10'-0''	8'-8''	6'-10''	6'-0''	
			_	=4 OL OIL =	4.01.01!	OF 4 OF	-111	=clatt		

TABLE NOTES:

- 2015 ALUMINUM DESIGN MANUAL, ALLOWABLE STRESS DESIGN METHOD USED IN ALL TABLES.
- USE APPROPRIATE TABLE REQUIRED BY THE GOVERNING BUILDING CODE. VERIFY REQUIREMENTS WITH BUILDING DEPARTMENT.

 DEFLECTION LIMIT = L/90.
- COLUMN SPACING IS HALF THE DISTANCE TO THE LEFT ADDED TO HALF THE DISTANCE TO THE RIGHT OF THE BEAM (AVERAGE COLUMN SPACING).
 VALUES BELOW ALLOWABLE CEILING HEIGHT INTENDED TO BE BUILT ON KNEEWALLS
- OR OTHER SUPPORTING STRUCTURES (CERTIFIED BY OTHERS). VALUES AND USE FOR SINGLE SPAN PROJECTION FROM STRUCTURE ONLY.



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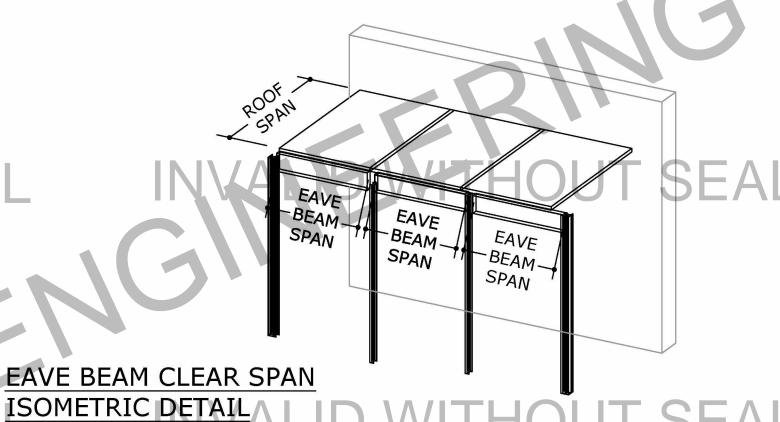


EAVE BEAM SPAN TABLES:

BEAM	ROOF LOAD	MAX BEAM SPAN						
TYPE	(PSF)	8	10	12	14	16		
	/A ²⁰ ID	11'-1"	10'-1''	9'-4''	8'-9''	8'-3''		
		11'-1"	10'-1''	9'-4''	8'-9''	8'-3"		
A	30 40	9'-1''	8'-3"	7'-8''	7'-2''	6'-9''		
		9'-1"	8'-3"	7'-8''	7'-2"	6'-9''		
AAF 2x6		7'-10''	7'-2"	6'-7''	6'-2''	5'-10''		
¥		7'-10''	7'-2''	6'-7''	6'-2''	5'-10''		
	F.O.	7'-0''	6'-5''	6'-0''	5'-6''	5'-3''		
	50	7'-0''	6'-5"	6'-0''	5'-6''	5'-3''		

BEAM	BEAM ROOF LOAD		MAX BEAM SPAN						
TYPE	(PSF)	8	10	12	14	16			
	20	12'-1"	11'-1"	10'-3"	9'-7''	9'-0''			
		12'-1"	11'-1"	10'-3"	9'-7''	9'-0''			
ΑP	20	9'-11"	9'-0''	8'-4''	7'-10''	7'-5''			
AAF 2x7 LAP	30	9'-11"	9'-0''	8'-4''	7'-10''	7'-5''			
	40	8'-7''	7'-10''	7'-3''	6'-9''	6'-5"			
	40	8'-7''	7'-10"	7'-3''	6'-9''	6'-5"			
11/11/	/ A to I D	7'-8''	7'-0''	6'-6''	6'-1"	5'-9"			
IIV	/A ⁵⁰ ID	7'-8"	7'-0"	6'-6"	6'-1"	5'-9"			

BEAM	ROOF LOAD	MAX BEAM SPAN					
TYPE	(PSF)	8	10	12	14	16	
AAF 2x6 LAP	20	15'-0"	15'-0''	15'-0''	15'-0''	14'-1''	
		15'-0"	15'-0''	15'-0''	15'-0''	14'-1''	
	30	15'-0"	14'-1''	13'-0''	12'-2"	11'-6''	
		15'-0"	14'-1''	13'-0"	12'-2''	-11'-6"	
	/A ₄₀ ID	13'-4"	12'-2''	11'-3"	10'-7''	10'-0"	
		13'-4"	12'-2"	11'-3"	10'-7''	10'-0''	
	50	12'-0''	10'-11''	10'-1''	9'-5''	8'-11''	
		12'-0''	10'-11''	10'-1"	9'-5''	8'-11''	



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